Memorandum

TO: Town of Lysander Comprehensive Plan Update Committee

FROM: Meghan Vitale, SMTC

DATE: October 14, 2014

RE: Existing conditions assessment (Technical Memorandum #3)

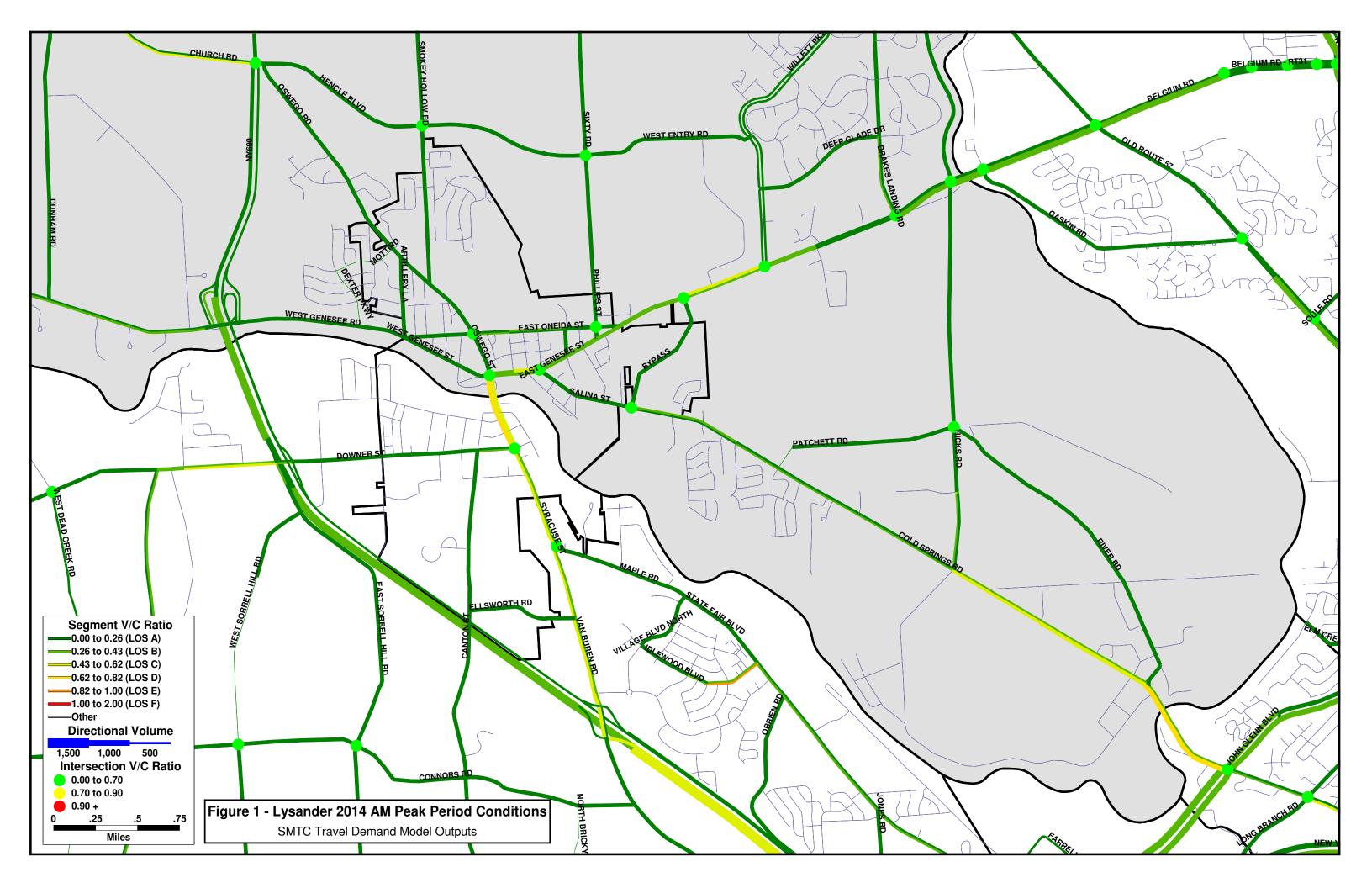
The SMTC has reviewed the outputs from our travel demand model for various intersections and segments within the Town of Lysander and conducted capacity analysis for selected intersections based on recent traffic count data. This memo summarizes the work conducted and our findings.

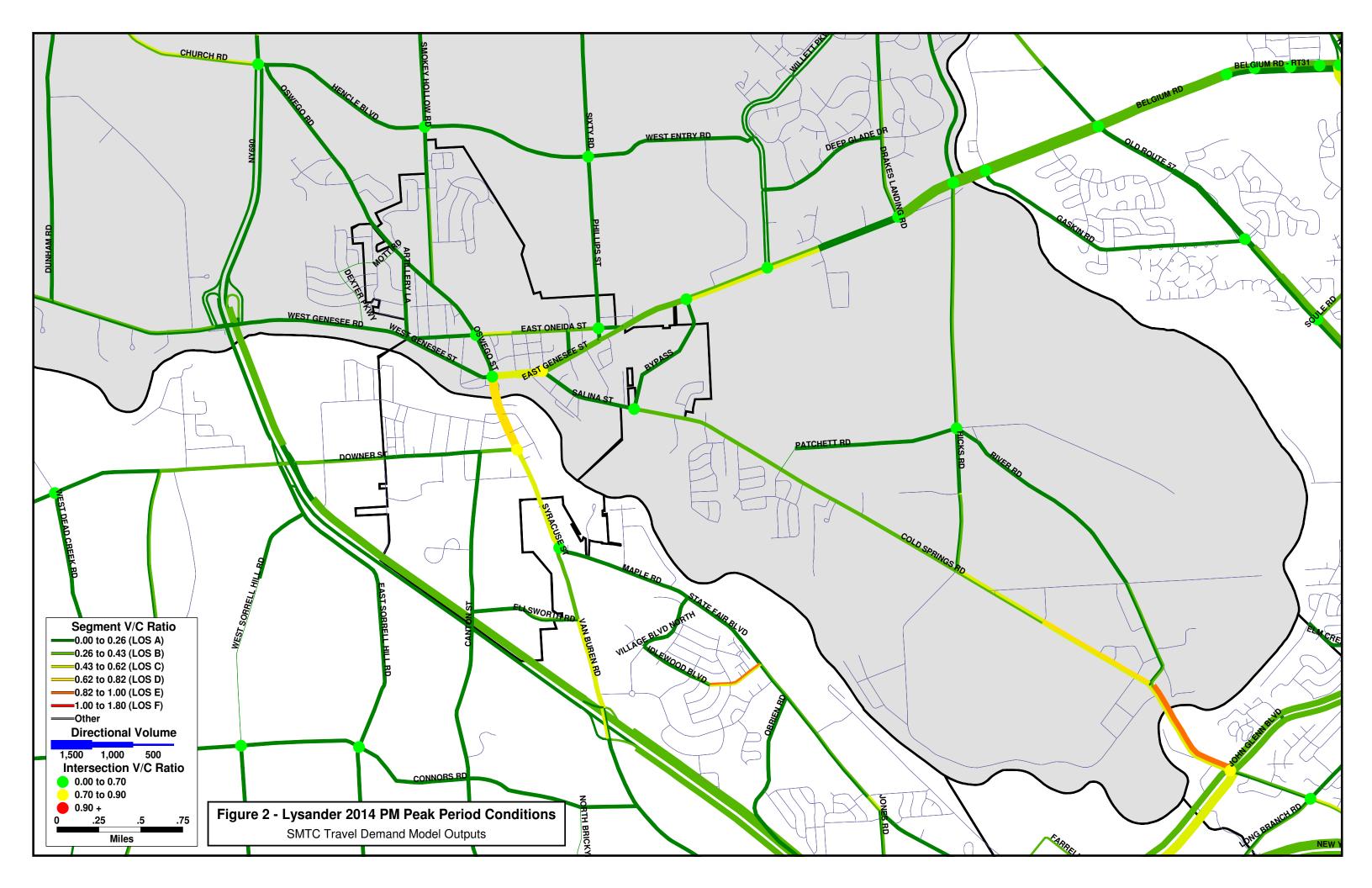
Review of volume-to-capacity ratios from SMTC's travel demand model

SMTC staff reviewed the volume-to-capacity ratios from the 2014 Existing Conditions travel demand model for road segments and intersections throughout the Town of Lysander.

The volume-to-capacity ratio varies from 0.0 to 1.0 and is a comparison of the current or projected traffic on a road or entering an intersection (in this case, the modeled volume) to the maximum traffic volume that the segment or intersection can reasonably be expected to accommodate. Values closer to 1.0 indicate that a segment or intersection is approaching capacity and operational issues may occur (drivers will experience longer delays). The volume-to-capacity ratio is abbreviated "V/C".

Figures 1 and 2 show the V/C ratio for model links (segments) and nodes (intersections) within the Cold Springs Peninsula area of the Town of Lysander and part of the adjoining towns for the AM and PM peak hours, respectively. Both figures show the 2014 Existing Conditions based on the SMTC travel demand model outputs.





All modeled intersections within the town except one have existing V/C ratios below 0.70, indicating good operation conditions. The intersection of Route 31and Route 370 (East Genesee Street and Salina Street in the Village of Baldwinsville) is the only intersection with a V/C ratio between 0.7 and 0.9 (and only during the PM peak hour), indicating that drivers likely experience longer delays at this intersection.

The vast majority of modeled road segments show very good existing conditions (V/C ratios less than 0.43, or likely a level of service [LOS] A or B). The segments with higher V/C ratios include Route 31 near Willet Parkway, Route 370 between Hicks Road and River Road, Route 48 south of Route 31/370, and Route 31/370 east of Route 48. However, the V/C ratios for these segments are still relatively low (less than 0.82) indicating that these roads generally experience acceptable operating conditions (LOS C or D). The only road segment that appears to be approaching capacity is Route 370 between River Road and John Glenn Boulevard in the PM peak hour, which carries nearly 1,500 vehicles in the PM peak hour based on the travel demand model. Although Route 31 near the boundary with the Town of Clay carries the highest traffic volume in the town (even more than Route 690), this segment has two travel lanes in each direction and, therefore, has a higher capacity than Route 370.

Existing conditions intersection capacity analysis

SMTC also conducted capacity analysis at selected intersections to gain more detailed understanding of existing traffic operations at these locations.

Technical Memorandum #1 (August 4, 2014) summarized all of the recent turning movement counts that were conducted by the SMTC, the NYSDOT, and consultants. SMTC staff conducted turning movement counts at five intersections, and conducted a capacity analysis for each of these intersections under AM and PM peak hour conditions. The capacity analysis was conducted using Synchro 7 software. The resulting LOS and delay information is summarized in Table 1 and Table 2 for signalized and unsignalized intersections, respectively. Tables 1 and 2 also include level of service information from the "Timber Banks with YMCA Traffic Assessment – Response to NYSDOT Comments" (GTS Consulting, March 2013) for an additional four intersections. Attachment A contains the reports from Synchro for the five intersections analyzed by the SMTC.

Intersection Approach	Movement	AM peak hour LOS (delay)	PM peak hour LOS (delay)	Source
Route 690 / Her	ncle Boulevard	B (16)	B (15)	SMTC
Eastbound	Left/through/right	B (14)	B (19)	
Westbound	Left/through	C (23)	C (24)	
	Right	B (13)	B (18)	
Northbound	Left	C (28)	C (24)	
	Through	B (13)	B (11)	
	Right	B (12)	A (9)	
Southbound	Left	C (27)	C (23)	
	Through	B (15)	B (12)	
	Right	B (12)	B (10)	
Route 370 / Rou	ıte 48	B (15)	B (18)	SMTC
Eastbound	Left/through	C (27)	D (38)	
	Right	C (43)	C (32)	
Westbound	Left	B (13)	B (17)	
	Through/right	A (7)	A (10)	
Northbound	Left/through	C (22)	C (28)	
	Right	A (6)	A (7)	
Southbound	Left/through/right	C (22)	C (24)	
Route 370 / Rou	ıte 631	A (7)	A (7)	SMTC
Eastbound	Left	A (5)	A (5)	
	Through	A (6)	A (7)	
Westbound	Through	A (6)	A (8)	
	Right	A (2)	A (3)	
Southbound	Left	B (15)	B (12)	
	Right	B (13)	B (11)	
Route 31 / Rive	r Road	C (22)	C (27)	GTS
Eastbound	Left	D (43)	D (43)	
	Through	C (21)	C (30)	
	Right	A (5)	A (7)	
Westbound	Left	D (45)	E (57)	
	Through/right	B (14)	B (19)	
Northbound	Left	C (22)	C (28)	
	Through	D (38)	D (37)	
	Right	C (25)	C (22)	
Southbound	Left	C (29)	C (24)	
	Through/Right	D (36)	C (33)	

Table 1: Summary of Existing Conditions Capacity Analysis, Signalized Intersection	Table 1: Summary	of Existing Conditions	Capacity Analysis,	Signalized Intersections
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LOS = Level of service. Delay is the average delay per vehicle, in seconds. Note: Bold text indicates the overall LOS and average delay for a signalized intersection.

Source: SMTC, 2014; "Timber Banks with YMCA Traffic Assessment – Response to NYSDOT Comments" by GTS Consulting, March 2013.

Intersection		AM peak hour	PM peak hour	
Approach	Movement	LOS (delay)	LOS (delay)	Source
Route 370 / Route	e 690 SB ramps			SMTC
Eastbound	Left	B (10)	B (10)	
Southbound	Left/right	C (23)	C (20)	
Route 370 / Route	e 690 NB ramps			SMTC
Eastbound	Left	A (8)	A (8)	
Southbound	Left	B (15)	C (21)	
	Right	B (12)	C (21)	
Route 370 / Hicks	s Road / Hayes Road			GTS
Eastbound	Left/through/right	A (1)	A (1)	
Westbound	Left/through/right	A (1)	A (1)	
Northbound	Left/through/right	B (15)	C (21)	
Southbound	Left/through/right	F (69)	D (35)	
Route 370 / River	Road			GTS
Eastbound	Left/through	A (1)	A (1)	
Westbound	Through/right	A (0)	A (0)	
Southbound	Left/right	F (95)	F (193)	
River Road / Pate	chett Road / Hicks Road	1		GTS
Northbound	Left/through/right	A (5)	A (6)	
Northwestbound	Left/through/right	A (4)	A (5)	
Southbound	Left/through/right	A (7)	A (9)	
Eastbound	Left/through/right	A (5)	A (6)	
Westbound	Left/through/right	A (3)	A (3)	

Table 2: Summary of Existing Conditions Capacity Analysis, Unsignalized Intersections

LOS = Level of service. Delay is the average delay per vehicle, in seconds. Note: For unsignalized intersections, the HCM methodology does not provide an overall LOS or delay. Source: SMTC, 2014; "Timber Banks with YMCA Traffic Assessment – Response to NYSDOT Comments" by GTS Consulting, March 2013.

The signalized intersections that were examined all operate at good overall levels of service with minimal to moderate delay. Most individual movements operate at LOS A or B with minimal delay. The Route 31/River Road intersection operates with the greatest delay of the intersections considered, although it still operates at an overall LOS C. Some individual movements at this intersection experience greater delay, operating at LOS D or E. The westbound left turn movement from Route 31 to River Road experiences the highest delay – at 57 seconds and LOS E - of any of the movements examined for signalized intersections.

In 2013, the NYSDOT conducted capacity analysis for the Route 370/John Glenn Boulevard intersection in the Town of Salina. This analysis indicates that the intersection operates at an

overall LOS F during the AM peak hour and LOS E during the PM peak hour, with the following individual movements operating at LOS F:

- Route 370 eastbound left (AM)
- Route 370 eastbound through/right (AM and PM)
- Route 370 westbound through/right (PM)
- John Glenn Boulevard northbound left (PM)

Turning movements at the unsignalized intersections that were examined mostly operate at LOS A, B, or C, indicating relatively good operations with little delay to drivers. The exceptions are the southbound movements from River Road to Route 370 and the southbound movements from Hicks Road to Route 370, which operate at LOS D or F. At both of these locations, there is a single southbound lane from which to make all turning movements onto Route 370 and most of the southbound traffic on Hicks Road and River Road turns left onto Route 370. The southbound left turn volume at both of these intersections is approximately 200 vehicles in the AM peak hour and 100 vehicles in the PM peak hour, and the volumes on Route 370 vary from around 700 vehicles per hour to over 1,300 vehicles per hour. The result is that drivers trying to turn onto Route 370 from Hicks Road or River Road may experience delay, on average, of 1 to 3 minutes. According to the "Timber Banks with YMCA Traffic Assessment – Response to NYSDOT Comments" (GTS Consulting, March 2013), both of these locations may be considered for a signal warrant analysis once the reduced initial build out levels for Timber Banks are reached.

Summary

In general, the results of our intersection capacity analysis and review of the travel demand model outputs for segments and intersections indicate that traffic flows quite well in the Town of Lysander under the existing conditions. There are a few isolated areas of moderate concern such as Routes 31/370 and Route 48 near the "four corners" in Baldwinsville and Route 31 near Willett Parkway which may begin to approach capacity in the future, depending on the level of development and amount of new trips that are generated. There are also a few places that currently experience "poor" levels of service. These locations include the unsignalized intersections of Hicks Road and River Road with Route 370, the westbound left-turn movement from Route 31 onto River Road, and Route 370 between River Road and John Glenn Boulevard. Future development plans should be mindful of the potential for additional traffic at these locations, which would exacerbate the existing concerns. Mitigation measures, such as new traffic signals, may need to be considered as part of future development plans; however, this would require additional technical analysis and coordination with the NYSDOT. The cost of installing and maintaining a traffic signal - or any transportation infrastructure that increases capacity – should be weighed against the likely benefit to all residents and the potential to induce additional demand in the future.

Attachment A

Synchro reports

HCM Signalized Intersection Capacity Analysis 1: Church Road & Route 48

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			र्स	1	ሻ	- ††	1	<u>۲</u>	- ††	1
Volume (vph)	5	44	127	158	17	38	33	228	67	41	444	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0			6.0	6.0	5.0	7.0	7.0	5.0	7.0	7.0
Lane Util. Factor		1.00			1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.90			1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		1.00			0.96	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1667			1663	1615	1492	3312	1429	1687	3374	1615
Flt Permitted		0.99			0.65	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1649			1138	1615	1492	3312	1429	1687	3374	1615
Peak-hour factor, PHF	0.81	0.81	0.81	0.78	0.78	0.78	0.83	0.83	0.83	0.90	0.90	0.90
Adj. Flow (vph)	6	54	157	203	22	49	40	275	81	46	493	3
RTOR Reduction (vph)	0	114	0	0	0	35	0	0	55	0	0	2
Lane Group Flow (vph)	0	103	0	0	225	14	40	275	26	46	493	1
Heavy Vehicles (%)	0%	5%	2%	9%	12%	0%	21%	9%	13%	7%	7%	0%
Turn Type	Perm			Perm		Perm	Prot		Perm	Prot		Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8			2			6
Actuated Green, G (s)		13.9			13.9	13.9	2.5	15.9	15.9	2.5	15.9	15.9
Effective Green, g (s)		13.9			13.9	13.9	2.5	15.9	15.9	2.5	15.9	15.9
Actuated g/C Ratio		0.28			0.28	0.28	0.05	0.32	0.32	0.05	0.32	0.32
Clearance Time (s)		6.0			6.0	6.0	5.0	7.0	7.0	5.0	7.0	7.0
Vehicle Extension (s)		2.0			2.0	2.0	2.0	5.7	5.7	2.0	5.7	5.7
Lane Grp Cap (vph)		456			314	446	74	1047	452	84	1067	511
v/s Ratio Prot							0.03	0.08		c0.03	c0.15	
v/s Ratio Perm		0.06			c0.20	0.01			0.02			0.00
v/c Ratio		0.23			0.72	0.03	0.54	0.26	0.06	0.55	0.46	0.00
Uniform Delay, d1		14.1			16.4	13.3	23.3	12.8	12.0	23.3	13.8	11.8
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		0.1			6.4	0.0	4.3	0.3	0.1	3.9	0.8	0.0
Delay (s)		14.1			22.8	13.3	27.6	13.2	12.1	27.2	14.6	11.8
Level of Service		В			С	В	С	В	В	С	В	В
Approach Delay (s)		14.1			21.1			14.4			15.7	
Approach LOS		В			С			В			В	
Intersection Summary												
HCM Average Control Delay			16.1	Н	CM Leve	l of Servic	e		В			
HCM Volume to Capacity ratio			0.58									
Actuated Cycle Length (s)			50.3	S	um of los	t time (s)			18.0			
Intersection Capacity Utilization	n		55. 9 %	IC	CU Level	of Service			В			
Analysis Period (min)			15									
c Critical Lano Croup												

c Critical Lane Group

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	۲	↑	≜	1	¥	
Volume (veh/h)	351	188	185	163	10	17
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.84	0.84	0.79	0.79	0.75	0.75
Hourly flow rate (vph)	418	224	234	206	13	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	441				1294	234
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	441				1294	234
tC, single (s)	4.1				6.5	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.6	3.3
p0 queue free %	62				88	97
cM capacity (veh/h)	1114				108	810
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	418	224	234	206	36	
Volume Left	418	0	0	0	13	
Volume Right	0	0	0	206	23	
cSH	1114	1700	1700	1700	238	
Volume to Capacity	0.38	0.13	0.14	0.12	0.15	
Queue Length 95th (ft)	44	0	0	0	13	
Control Delay (s)	10.2	0.0	0.0	0.0	22.8	
Lane LOS	B				С	
Approach Delay (s)	6.6		0.0		22.8	
Approach LOS					С	
Intersection Summary						
Average Delay			4.5			
Intersection Capacity Utiliz	ation		42.5%	IC	U Level o	of Service
Analysis Period (min)			15			

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	۲	1		1	٦	1
Volume (veh/h)	18	187	242	10	61	111
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.93	0.93	0.69	0.69	0.78	0.78
Hourly flow rate (vph)	19	201	351	14	78	142
Pedestrians	.,	201				
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)		None	None			
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	365				591	351
vC1, stage 1 conf vol	505				571	551
vC2, stage 2 conf vol						
vCu, unblocked vol	365				591	351
tC, single (s)	4.1				6.5	6.3
tC, 2 stage (s)	т. і				0.5	0.5
tF (s)	2.2				3.6	3.4
p0 queue free %	98				82	79
cM capacity (veh/h)	1204				445	677
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Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volume Total	19	201	351	14	78	142
Volume Left	19	0	0	0	78	0
Volume Right	0	0	0	14	0	142
cSH	1204	1700	1700	1700	445	677
Volume to Capacity	0.02	0.12	0.21	0.01	0.18	0.21
Queue Length 95th (ft)	1	0	0	0	16	20
Control Delay (s)	8.0	0.0	0.0	0.0	14.8	11.7
Lane LOS	А				В	В
Approach Delay (s)	0.7		0.0		12.8	
Approach LOS					В	
Intersection Summary						
Average Delay			3.7			
Intersection Capacity Utiliz	ation		26.3%	IC	U Level o	of Service
Analysis Period (min)			15			
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HCM Signalized Intersection Capacity Analysis 4: Route 370/Route 31 & Route 48

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र् ग	1	<u>۲</u>	ef 👘			र्च	1		4	
Volume (vph)	4	187	86	436	134	28	43	130	432	3	179	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	10	12	11	13	12	12	13	12	16	12
Total Lost time (s)		5.0	5.0	5.0	5.0			5.5	5.5		5.5	
Lane Util. Factor		1.00	1.00	1.00	1.00			1.00	1.00		1.00	
Frpb, ped/bikes		1.00	0.98	1.00	1.00			1.00	1.00		1.00	
Flpb, ped/bikes		1.00	1.00	1.00	1.00			1.00	1.00		1.00	
Frt		1.00	0.85	1.00	0.97			1.00	0.85		1.00	
Flt Protected		1.00	1.00	0.95	1.00			0.99	1.00		1.00	
Satd. Flow (prot)		1774	1276	1735	1674			1546	1636		1733	
Flt Permitted		0.99	1.00	0.40	1.00			0.87	1.00		1.00	
Satd. Flow (perm)		1763	1276	735	1674			1368	1636		1726	
Peak-hour factor, PHF	0.95	0.95	0.95	0.85	0.85	0.85	0.89	0.89	0.89	0.77	0.77	0.77
Adj. Flow (vph)	4	197	91	513	158	33	48	146	485	4	232	9
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	201	91	513	191	0	0	194	485	0	245	0
Confl. Peds. (#/hr)	1		1	1		1	4		2	2		4
Heavy Vehicles (%)	25%	3%	4%	4%	7%	4%	7%	26%	2%	0%	10%	43%
Parking (#/hr)			0								0	
Turn Type	Perm		Perm	pm+pt			pm+pt		pt+ov	Perm		
Protected Phases		2		1	6		7	4	41		8	
Permitted Phases	2		2	6			4			8		
Actuated Green, G (s)		13.9	13.9	38.9	38.9			19.1	44.6		19.1	
Effective Green, g (s)		13.9	13.9	38.9	38.9			19.1	44.6		19.1	
Actuated g/C Ratio		0.20	0.20	0.57	0.57			0.28	0.65		0.28	
Clearance Time (s)		5.0	5.0	5.0	5.0			5.5			5.5	
Vehicle Extension (s)		3.0	3.0	3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		358	259	709	951			381	1065		481	
v/s Ratio Prot				c0.21	0.11				c0.30			
v/s Ratio Perm		0.11	0.07	c0.20				0.14			0.14	
v/c Ratio		0.56	0.35	0.72	0.20			0.51	0.46		0.51	
Uniform Delay, d1		24.6	23.4	9.7	7.2			20.8	5.9		20.8	
Progression Factor		1.00	1.00	1.00	1.00			1.00	1.00		1.00	
Incremental Delay, d2		2.0	0.8	3.7	0.1			1.1	0.3		0.9	
Delay (s)		26.6	24.3	13.4	7.3			21.8	6.2		21.6	
Level of Service		С	С	В	A			С	A		С	
Approach Delay (s)		25.9			11.7			10.7			21.6	
Approach LOS		С			В			В			С	
Intersection Summary												
HCM Average Control Delay			14.8	H	CM Level	of Servic	e		В			
HCM Volume to Capacity ratio			0.64									
Actuated Cycle Length (s)			68.5	S	um of lost	time (s)			10.5			
Intersection Capacity Utilization	1		76.3%		CU Level of)		D			
Analysis Period (min)			15		5 200010				U			
c Critical Lane Group												
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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	ሻ	†		1	٦	1	
Volume (vph)	8	236	232	79	74	2	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width	12	12	10	12	12	12	
Total Lost time (s)	6.5	6.5	6.5	6.0	6.0	6.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1597	1810	1689	1495	1626	1615	
Flt Permitted	0.60	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1003	1810	1689	1495	1626	1615	
Peak-hour factor, PHF	0.80	0.80	0.88	0.88	0.71	0.71	
Adj. Flow (vph)	10	295	264	90	104	3	
RTOR Reduction (vph)	0	0	0	0	0	3	
Lane Group Flow (vph)	10	295	264	90	104	0	
Heavy Vehicles (%)	13%	5%	5%	8%	11%	0%	
Turn Type	Perm		-	pm+ov		Perm	
Protected Phases		6	2	4	4		
Permitted Phases	6	10.0	10.0	2		4	
Actuated Green, G (s)	18.8	18.8	18.8	24.2	5.4	5.4	
Effective Green, g (s)	18.8	18.8	18.8	24.2	5.4	5.4	
Actuated g/C Ratio	0.51	0.51	0.51	0.66	0.15	0.15	
Clearance Time (s)	6.5	6.5	6.5	6.0	6.0	6.0	
Vehicle Extension (s)	6.0	6.0	6.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	514	927	865	1230	239	238	
v/s Ratio Prot	0.01	c0.16	0.16	0.01	c0.06	0.00	
v/s Ratio Perm	0.01	0.22	0.01	0.05	0.44	0.00	
v/c Ratio	0.02	0.32	0.31	0.07	0.44	0.00	
Uniform Delay, d1	4.4	5.2	5.2 1.00	2.2	14.3	13.4	
Progression Factor	1.00 0.0	1.00	0.6	1.00 0.0	1.00 0.5	1.00 0.0	
Incremental Delay, d2	0.0 4.5	0.6 5.8	0.8 5.7	2.2	0.5 14.7	13.4	
Delay (s) Level of Service	4.5 A	о.с А		2.2 A	14.7 B	13.4 B	
Approach Delay (s)	А	5.7	A 4.9	A	ь 14.7	D	
Approach LOS		5.7 A	4.9 A		14.7 B		
		A	A		D		
Intersection Summary							
HCM Average Control Delay			6.6	Н	CM Level	of Service	
HCM Volume to Capacity rat	io		0.34				
Actuated Cycle Length (s)			36.7		um of lost		
Intersection Capacity Utilizat	ion		30.4%	IC	CU Level o	of Service	
Analysis Period (min)			15				
c Critical Lane Group							

HCM Signalized Intersection Capacity Analysis 1: Church Road & Route 48

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			ę	1	ľ	<u></u>	1	ľ	<u></u>	7
Volume (vph)	2	36	55	89	35	43	121	534	155	62	301	3
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		6.0			6.0	6.0	5.0	7.0	7.0	5.0	7.0	7.0
Lane Util. Factor		1.00			1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.92			1.00	0.85	1.00	1.00	0.85	1.00	1.00	0.85
Flt Protected		1.00			0.97	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)		1746			1656	1615	1787	3539	1442	1770	3282	1615
Flt Permitted		0.99			0.81	1.00	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)		1728			1382	1615	1787	3539	1442	1770	3282	1615
Peak-hour factor, PHF	0.69	0.69	0.69	0.92	0.92	0.92	0.92	0.92	0.92	0.89	0.89	0.89
Adj. Flow (vph)	3	52	80	97	38	47	132	580	168	70	338	3
RTOR Reduction (vph)	0	67	0	0	0	40	0	0	100	0	0	2
Lane Group Flow (vph)	0	68	0	0	135	7	132	580	68	70	338	1
Heavy Vehicles (%)	0%	0%	0%	15%	0%	0%	1%	2%	12%	2%	10%	0%
Turn Type	Perm			Perm		Perm	Prot		Perm	Prot		Perm
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8			2			6
Actuated Green, G (s)		7.9			7.9	7.9	6.1	20.3	20.3	4.0	18.2	18.2
Effective Green, g (s)		7.9			7.9	7.9	6.1	20.3	20.3	4.0	18.2	18.2
Actuated g/C Ratio		0.16			0.16	0.16	0.12	0.40	0.40	0.08	0.36	0.36
Clearance Time (s)		6.0			6.0	6.0	5.0	7.0	7.0	5.0	7.0	7.0
Vehicle Extension (s)		2.0			2.0	2.0	2.0	5.7	5.7	2.0	5.7	5.7
Lane Grp Cap (vph)		272			217	254	217	1431	583	141	1190	586
v/s Ratio Prot							c0.07	c0.16		0.04	0.10	
v/s Ratio Perm		0.04			c0.10	0.00			0.05			0.00
v/c Ratio		0.25			0.62	0.03	0.61	0.41	0.12	0.50	0.28	0.00
Uniform Delay, d1		18.5			19.8	17.9	20.9	10.7	9.3	22.1	11.4	10.2
Progression Factor		1.00			1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2		0.2			4.0	0.0	3.3	0.5	0.2	1.0	0.3	0.0
Delay (s)		18.7			23.7	17.9	24.2	11.1	9.6	23.1	11.7	10.2
Level of Service		В			С	В	С	В	А	С	В	В
Approach Delay (s)		18.7			22.2			12.8			13.6	
Approach LOS		В			С			В			В	
Intersection Summary												
HCM Average Control Delay			14.6	Н	CM Leve	l of Servic	e		В			
HCM Volume to Capacity ratio			0.43									
Actuated Cycle Length (s)			50.2	S	um of los	t time (s)			11.0			
Intersection Capacity Utilization	n		47.6%			of Service	<u>;</u>		А			
Analysis Period (min)			15									
c Critical Lano Group												

c Critical Lane Group

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	٦	1	•	1	¥	
Volume (veh/h)	156	174	586	114	11	21
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.95	0.95	0.88	0.88	0.89	0.89
Hourly flow rate (vph)	164	183	666	130	12	24
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	795				1177	666
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	795				1177	666
tC, single (s)	4.1				6.5	6.3
tC, 2 stage (s)						
tF (s)	2.2				3.6	3.4
p0 queue free %	80				92	95
cM capacity (veh/h)	826				164	446
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	
Volume Total	164	183	666	130	36	
Volume Left	164	0	000	0	12	
Volume Right	0	0	0	130	24	
cSH	826	1700	1700	1700	280	
Volume to Capacity	0.20	0.11	0.39	0.08	0.13	
Queue Length 95th (ft)	18	0.11	0.37	0.00	11	
Control Delay (s)	10.4	0.0	0.0	0.0	19.7	
Lane LOS	10.4 B	0.0	0.0	0.0	C	
Approach Delay (s)	4.9		0.0		19.7	
Approach LOS	т.7		0.0		C	
Intersection Summary					Ŭ	
Average Delay			2.1			
Intersection Capacity Utiliza	tion			10		of Service
	1011		52.8%	IC	U Level (I Service
Analysis Period (min)			15			

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Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	ሻ	†	•	1	5	1
Volume (veh/h)	14	167	325	17	196	396
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.77	0.77	0.93	0.93	0.83	0.83
Hourly flow rate (vph)	18	217	349	18	236	477
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	368				603	349
vC1, stage 1 conf vol	500				200	
vC2, stage 2 conf vol						
vCu, unblocked vol	368				603	349
tC, single (s)	4.2				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.3				3.5	3.3
p0 queue free %	98				48	31
cM capacity (veh/h)	1164				455	694
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	SB 1	SB 2
Volume Total	18	217	349	18	236	477
Volume Left	18	0	349 0	0	230	477
	0	0	0	18	230	477
Volume Right cSH	0 1164	1700	1700	1700	455	477 694
Volume to Capacity	0.02	0.13	0.21	0.01	455 0.52	0.69
1 3		0.13		0.01	0.52 73	0.69
Queue Length 95th (ft) Control Delay (s)	1 8.1		0 0.0	0.0	21.1	
3 • • •		0.0	0.0	0.0		20.8 C
Lane LOS	A 0.6		0.0		C	U
Approach Delay (s) Approach LOS	0.0		0.0		20.9 C	
					ι L	
Intersection Summary						
Average Delay			11.4			·
Intersection Capacity Utilization	n		48.3%		III evel c	of Service
Analysis Period (min)	511		15.576	10	O LOVOI C	

HCM Signalized Intersection Capacity Analysis 4: Route 370/Route 31 & Route 48

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1	٦	et 🗧			ب ا	1		4	
Volume (vph)	4	200	84	519	182	45	80	183	576	1	184	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width	12	11	10	12	11	13	12	12	13	12	16	12
Total Lost time (s)		5.0	5.0	5.0	5.0			5.5	5.5		5.5	
Lane Util. Factor		1.00	1.00	1.00	1.00			1.00	1.00		1.00	
Frpb, ped/bikes		1.00	0.96	1.00	0.99			1.00	1.00		1.00	
Flpb, ped/bikes		1.00	1.00	1.00	1.00			1.00	1.00		1.00	
Frt		1.00	0.85	1.00	0.97			1.00	0.85		1.00	
Flt Protected		1.00	1.00	0.95	1.00			0.99	1.00		1.00	
Satd. Flow (prot)		1799	1307	1801	1742			1850	1652		1911	
Flt Permitted		0.99	1.00	0.33	1.00			0.79	1.00		1.00	
Satd. Flow (perm)		1789	1307	618	1742			1488	1652		1909	
Peak-hour factor, PHF	0.91	0.91	0.91	0.93	0.93	0.93	0.94	0.94	0.94	0.82	0.82	0.82
Adj. Flow (vph)	4	220	92	558	196	48	85	195	613	1	224	6
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	224	92	558	244	0	0	280	613	0	231	0
Confl. Peds. (#/hr)	6		8	6		8	6		11	11		6
Heavy Vehicles (%)	0%	2%	0%	0%	2%	0%	1%	1%	1%	0%	1%	0%
Parking (#/hr)			0								0	
Turn Type	Perm		Perm	pm+pt			pm+pt		pt+ov	Perm		
Protected Phases		2		1	6		7	4	4 1		8	
Permitted Phases	2		2	6			4			8		
Actuated Green, G (s)		16.2	16.2	49.5	49.5			26.8	60.6		26.8	
Effective Green, g (s)		16.2	16.2	49.5	49.5			26.8	60.6		26.8	
Actuated g/C Ratio		0.19	0.19	0.57	0.57			0.31	0.70		0.31	
Clearance Time (s)		5.0	5.0	5.0	5.0			5.5			5.5	
Vehicle Extension (s)		3.0	3.0	3.0	3.0			3.0			3.0	
Lane Grp Cap (vph)		334	244	738	993			459	1153		589	
v/s Ratio Prot				c0.25	0.14				0.37			
v/s Ratio Perm		0.13	0.07	c0.18				c0.19			0.12	
v/c Ratio		0.67	0.38	0.76	0.25			0.61	0.53		0.39	
Uniform Delay, d1		32.8	30.9	12.7	9.3			25.5	6.3		23.6	
Progression Factor		1.00	1.00	1.00	1.00			1.00	1.00		1.00	
Incremental Delay, d2		5.2	1.0	4.4	0.1			2.4	0.5		0.4	
Delay (s)		38.0	31.9	17.2	9.5			27.9	6.8		24.0	
Level of Service		D	С	В	А			С	А		С	
Approach Delay (s)		36.2			14.8			13.4			24.0	
Approach LOS		D			В			В			С	
Intersection Summary												
HCM Average Control Delay			18.2	H	CM Level	of Servic	e		В			
HCM Volume to Capacity ratio			0.69									
Actuated Cycle Length (s)			86.8	Si	um of lost	t time (s)			10.5			
Intersection Capacity Utilization	1		85.9%			of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

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Movement	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations	<u> </u>	1	1	1	1	1	
Volume (vph)	26	297	343	99	73	14	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width	12	12	10	12	12	12	
Total Lost time (s)	6.5	6.5	6.5	6.0	6.0	6.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	1.00	0.85	1.00	0.85	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1736	1881	1756	1583	1787	1615	
Flt Permitted	0.53	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	971	1881	1756	1583	1787	1615	
Peak-hour factor, PHF	0.78	0.78	0.88	0.88	0.99	0.99	
Adj. Flow (vph)	33	381	390	112	74	14	
RTOR Reduction (vph)	0	0	0	0	0	12	
Lane Group Flow (vph)	33	381	390	112	74	2	
Heavy Vehicles (%)	4%	1%	1%	2%	1%	0%	
Turn Type	Perm			pm+ov		Perm	
Protected Phases		6	2	4	4		
Permitted Phases	6			2		4	
Actuated Green, G (s)	13.0	13.0	13.0	17.6	4.6	4.6	
Effective Green, g (s)	13.0	13.0	13.0	17.6	4.6	4.6	
Actuated g/C Ratio	0.43	0.43	0.43	0.58	0.15	0.15	
Clearance Time (s)	6.5	6.5	6.5	6.0	6.0	6.0	
Vehicle Extension (s)	6.0	6.0	6.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	419	812	758	1241	273	247	
v/s Ratio Prot		0.20	c0.22	0.01	c0.04		
v/s Ratio Perm	0.03	0.47	0.54	0.06	0.07	0.00	
v/c Ratio	0.08	0.47	0.51	0.09	0.27	0.01	
Uniform Delay, d1	5.0	6.1	6.2	2.7	11.3	10.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.2 5.3	1.2 7.3	1.6 7.8	0.0 2.8	0.2 11.5	0.0 10.8	
Delay (s) Level of Service			7.8 A	2.8 A	н.5 В	10.8 B	
Approach Delay (s)	А	A 7.1	6.7	A	ь 11.4	D	
Approach LOS		7.1 A	0.7 A		н.4 В		
••		А	A		D		
Intersection Summary							
HCM Average Control Delay			7.3	Н	CM Level	of Service	
HCM Volume to Capacity rati	0		0.45				
Actuated Cycle Length (s)			30.1		um of lost		
Intersection Capacity Utilizati	on		37.0%	IC	CU Level o	of Service	
Analysis Period (min)			15				
c Critical Lane Group							